HIGH PERFORMANCE STEEL

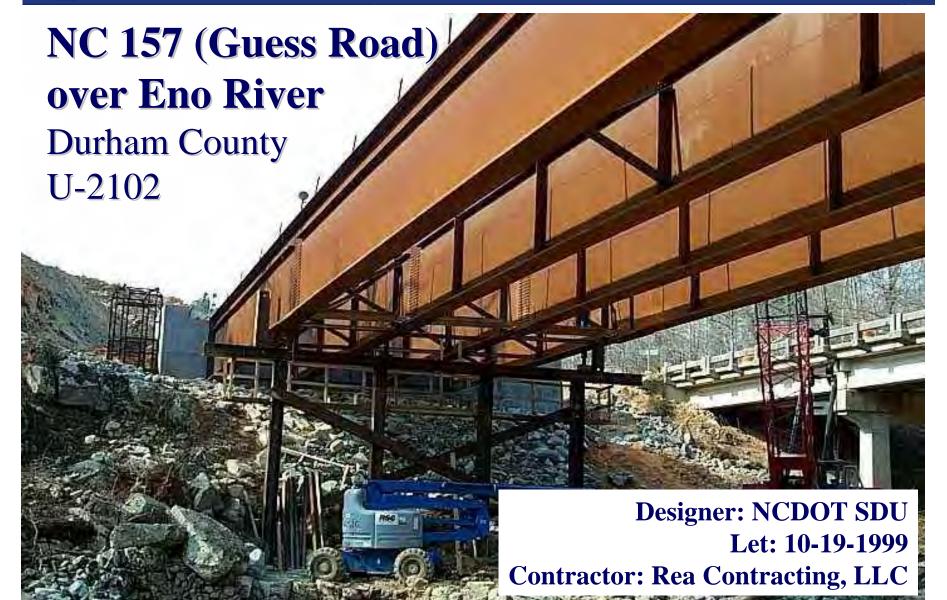
A Brief Overview of the use of HPS from the NCDOT Perspective

Jeff Vones, PE and Allen Raynor, PE



A Look at Three Projects

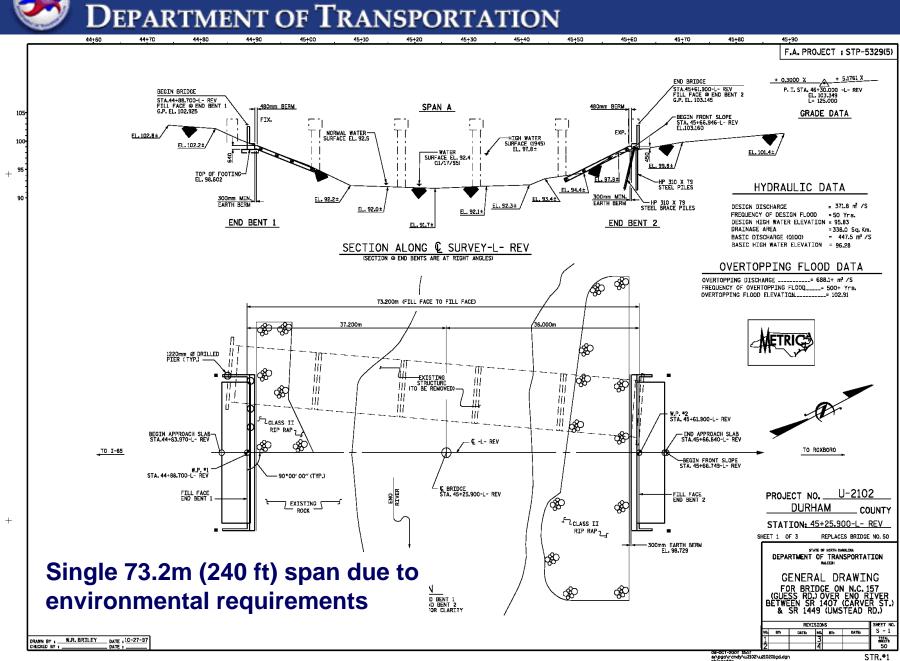
- NC 157 (Guess Road) over Eno River
 - U-2102 Durham County
- U.S. 64 over Hiwassee River & Martin Creek
 - R-0977A Cherokee County
- SR 1102 (Langtree Road) over I-77
 - I-4411 Iredell County



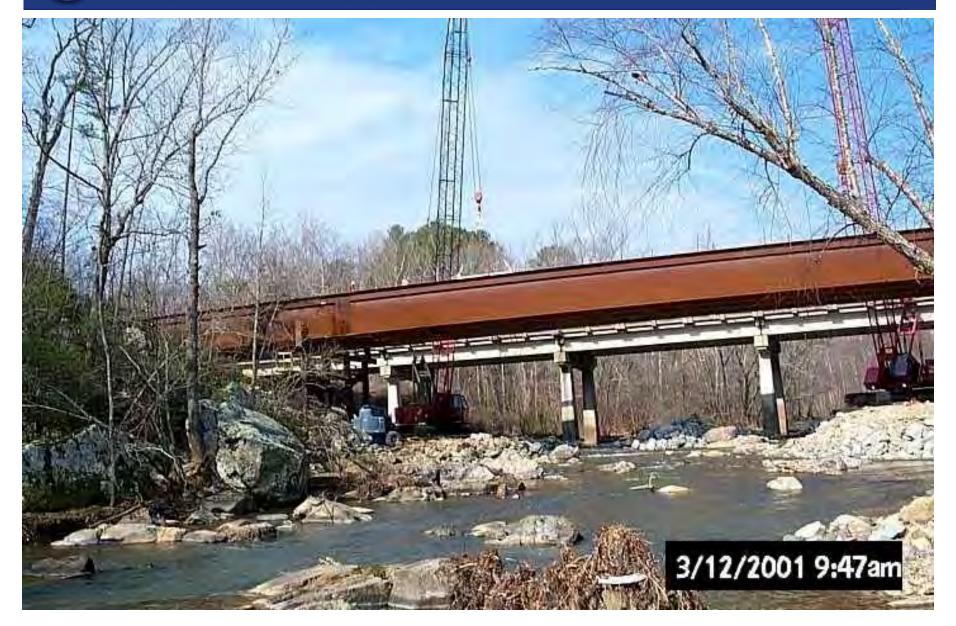
Project Data

- Single Span of 73.2m (240')
- Clear Roadway Width of 21.6m (70'-10")
 - w/ two 1.5m (5') Sidewalks
- Girder Spacing
 - Stage I: 3.24m (10'-7 1/2")
 - Stage II: 2.94m (9'-7 3/4")
- Girder Depth of 2.64m (8'-8")
- First use of HPS485W (70ksi) in NC

State of North Carolina DEPARTMENT OF TRANS

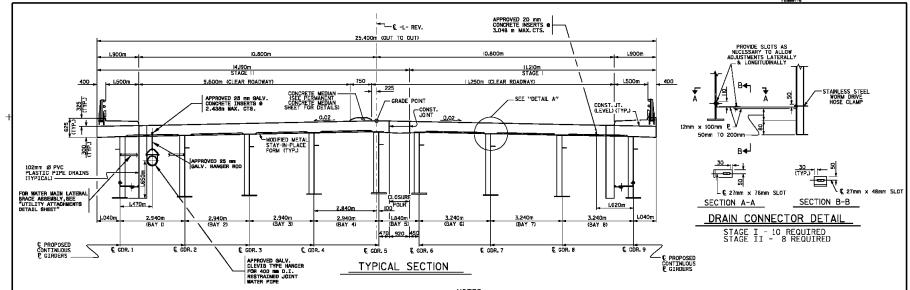


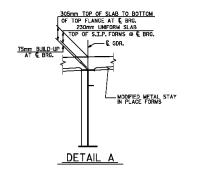






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PREVIOUSLY CAST CONCRETE SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 20.7 MPg BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT.

STRUCTURAL STEEL ERECTION SHALL BE COMPLETE BEFORE FALSEWORK OR FORMS ARE PLACED ON THE UNIT

*16 "G" BARS MAY BE SHIFTED SLIGHTLY AS NECESSARY TO CLEAR REINFORCING STEEL AND STIRRUPS.

FOR SAND LIGHTWEIGHT CONCRETE, SEE SPECIAL PROVISIONS. COUPLING IN DRAIN PIPE WILL BE PERMITTED AS APPROVED BY ENGINEER.

TOPS OF FLOOR DRAINS TO BE SET 10mm BELOW SURFACE OF SLAB.

4 - 13mm SQUARE LUGS TO BE GLUED TO THE PYC PLASTIC PIPE DRAIN AT EQUAL SPACES AROUND THE PIPE DRAIN AND APPROXIMATELY 100mm FROM THE TOP OF THE PIPE.

STAINLESS STEEL WORM DRIVE HOSE CLAMP SHALL BE OF COMMERCIAL QUALITY

152mm Ø PVC PLACTIC PIPE SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF

NO SEPARATE PAYMENT SHALL BE MADE FOR FURNISHING AND INSTALLING PYC PLASTIC PIPE DRAINS, THE COST OF THIS WORK SHALL BE INCLUDED IN THE PRICE BID FOR REINFORCED CONCRETE DECK SLAB (SAND LIGHTWEIGHT CONCRETE).

STAY-IN-PLACE METAL FORMS SHALL BE MODIFIED IN A MANNER THAT EXCLUDES CONCRETE ACCUMILATION IN THE FLUTE OF THE FORM POLISYTRENE WILL NOT BE PERMITTED FOR THIS PUMPOSE, SEE SPECIAL PROVISSIONS FOR MODIFIED METAL STAY-IN-PLACE FORMS.

PVC PLASTIC DECK DRAINS SHALL BE PAINTED WITH AN APPROVED VINYL PAINT, SEE SPECIAL PRVISIONS,

NOTES

BOLTS TO BE USED IN PIPE DRAIN CONNECTIONS SHALL BE OF THE SAME DIAMETER AND SHALL MEET THE SAME MATERIAL REQUIREMENTS AS THOSE SPECIFIED FOR THE DIAPHRAGM CONNECTIONS.

ALL DECK SLAB REINFORCING STEEL THAT IS FOUND TO CONFLICT WITH THE PROPER PLACEMENT OF THE 152mm DIA.PIPE DRAINS MAY BE CUT AS NECESSARY AT THE DIRECTION OF THE ENGINEER.

THE DIMENSIONS AND DETAILS SHOWN FOR THE 25mm AND 20mm CONCRETE INSERTS ARE FOR THE CONTRACTOR'S BENEFIT IN PLACING THEM AND SHOULD NOT BE CONSTRUED TO BE AN APPROVAL FOR THE ATTACHMENT OF THE UTILITY TO THE STRUCTURE.

FOR RESPONSIBILITIES FOR FURNISHING AND PLACING 25mm AND 20mm CONCRETE INSERTS, SEE THE UTILITY SPECIAL PROVISSIONS.

THE CONTRACTOR MAY WHEN INCESSARY PROPOSE A SCHEME FOR AVIOLING INTERFERING ENTWEM HETAL STAY-TH-PLACE FORM SUPPORTS OR FORMS AND BEAM-GIRDER STIFFENERS OR CONNECTOR PLATES, THE PROPOSAL SHALL BE INDICATED, AS APPROPATATE, ON EITHER THE STEEL WORKING DRAWINGS OR THE METAL STAY-IN-PLACE FORWARDING DRAWINGS

THE CONTRACTOR SHALL ADJUST THE GIRDER BUILD-UPS AS NECESSARY TO INCORPORATE A MAXIMUM PERMISSABLE VARIATION IN POT BEARING DEPTH OF 13mm, SEE SPECIAL PROVISION FOR POT BEARINGS.

DOWELS SHALL BE PLACED IN THE SAME HORIZONTAL PLANE AS THE TOP SLAB REINFORCING STEEL.

Staged Construction

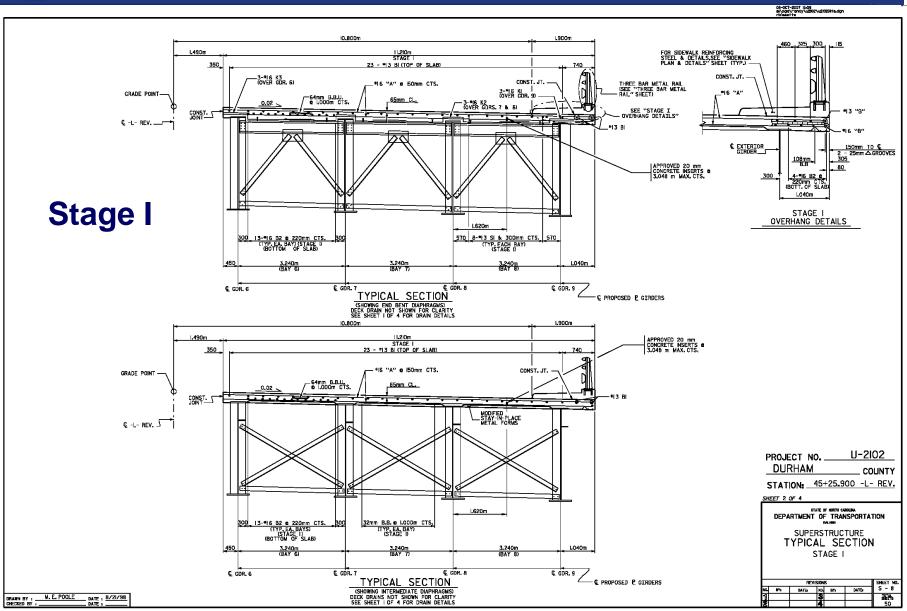
PROJECT NO. DURHAM COUNTY STATION: _45+25.900 -L- REV. SHEET | OF 4 DEPARTMENT OF TRANSPORTATION SUPERSTUCTURE TYPICAL SECTION REVISIONS S - 7 TOTAL 50

DRAWN BY : M. E. POOLE DATE : 7/17/98

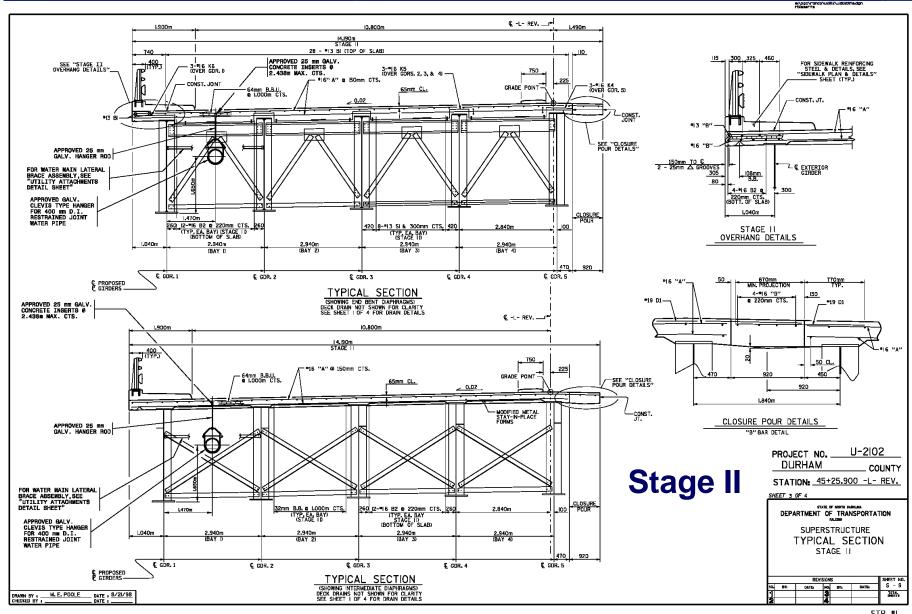
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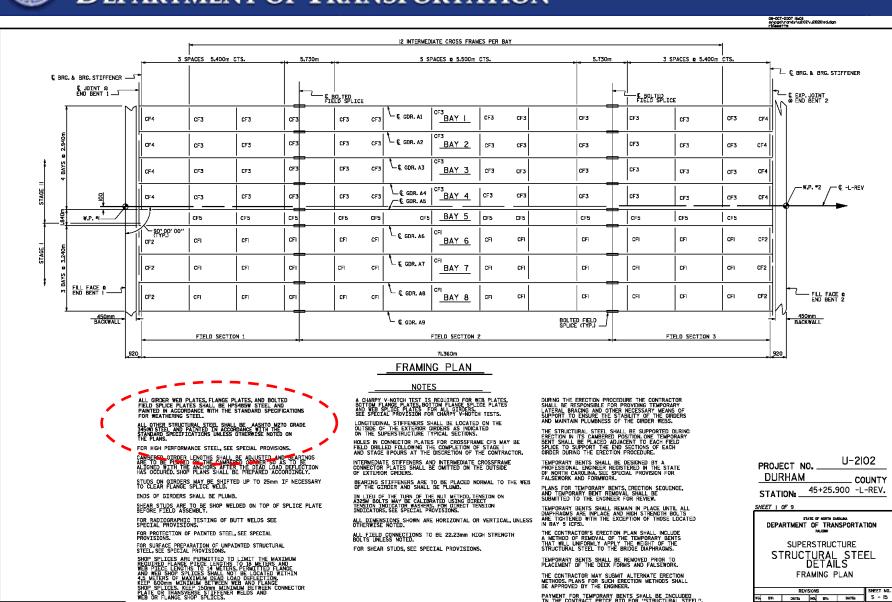






DRAWN BY : M. E. POOLE DATE : 8/20/98

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION



5 - 15

FRAMING PLAN

REVISIONS

BY: DATE NO BY: DATE:

THE CONTRACTOR MAY SUBMIT ALTERNATE ERECTION METHODS, PLANS FOR SUCH ERECTION METHODS SHALL BE APPROVED BY THE ENGINEER.

PAYMENT FOR TEMPORARY BENTS SHALL BE INCLUDED IN THE CONTRACT PRICE BID FOR "STRUCTURAL STEEL".

ALL GIRDER WEB PLATES, FLANGE PLATES, AND BOLTED FIELD SPLICE PLATES SHALL BE HPS485W STEEL AND PAINTED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR WEATHERING STEEL.

ALL OTHER STRUCTURAL STEEL SHALL BE AASHTO M270 GRADE 345(W) STEEL AND PAINTED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS UNLESS OTHERWISE NOTED ON THE PLANS.

FOR HIGH PERFORMANCE STEEL, SEE SPECIAL PROVISIONS.

CAMBERED GIRDER LENGTHS SHALL BE ADJUSTED AND BEARINGS ARE TO BE PLACED ON THE CAMBERED GIRDER SO AS TO BE ALIGNED WITH THE ANCHORS AFTER THE DEAD LOAD DEFLECTION HAS OCCURED. SHOP PLANS SHALL BE PREPARED ACCORDINGLY.

STUDS ON GIRDERS MAY BE SHIFTED UP TO 25mm IF NECESSARY TO CLEAR FLANGE SPLICE WELD.

ENDS OF GIRDERS SHALL BE PLUMB.

SHEAR STUDS ARE TO BE SHOP WELDED ON TOP OF SPLICE PLATE BEFORE FIELD ASSEMBLY.

FOR RADIOGRAPHIC TESTING OF BUTT WELDS SEE SPECIAL PROVISIONS.

FOR PROTECTION OF PAINTED STEEL, SEE SPECIAL PROVISIONS.

FOR SURFACE PREPARATION OF UNPAINTED STRUCTURAL STEEL, SEE SPECIAL PROVISIONS.

SHOP SPLICES ARE PERMITTED TO LIMIT THE MAXIMUM REQUIRED FLANGE PIECE LENGTHS TO 18 METERS AND WEB PIECE LENGTHS TO 14 METERS. PERMITTED FLANGE AND WEB SHOP SPLICES SHALL NOT BE LOCATED WITHIN 4.5 METERS OF MAXIMUM DEAD LOAD DEFLECTION. KEEP 600mm MINIMUM BETWEEN WEB AND FLANGE SHOP SPLICES. KEEP 150mm MINIMUM BETWEEN CONNECTOR PLATE OR TRANSVERSE STIFFENER WELDS AND WEB OR FLANGE SHOP SPLICES.

FRAMING PLAN

NOTES

A CHARPY V-NOTCH TEST IS REQUIRED FOR WEB PLATES, BOTTOM FLANGE PLATES, BOTTOM FLANGE SPLICE PLATES AND WEB SPLICE PLATES FOR ALL GIRDERS. SEE SPECIAL PROVISION FOR CHARPY V-NOTCH TESTS.

LONGITUDINAL STIFFENERS SHALL BE LOCATED ON THE OUTSIDE OF THE EXTERIOR GIRDERS AS INDICATED ON THE SUPERSTRUCTURE TYPICAL SECTIONS.

HOLES IN CONNECTOR PLATES FOR CROSSFRAME CF5 MAY BE FIELD DRILLED FOLLOWING THE COMPLETION OF STAGE I AND STAGE II POURS AT THE DISCRETION OF THE CONTRACTOR.

INTERMEDIATE STIFFENERS AND INTERMEDIATE CROSSFRAME CONNECTOR PLATES SHALL BE OMITTED ON THE OUTSIDE OF EXTERIOR GIRDERS.

BEARING STIFFENERS ARE TO BE PLACED NORMAL TO THE WEB OF THE GIRDER AND SHALL BE PLUMB.

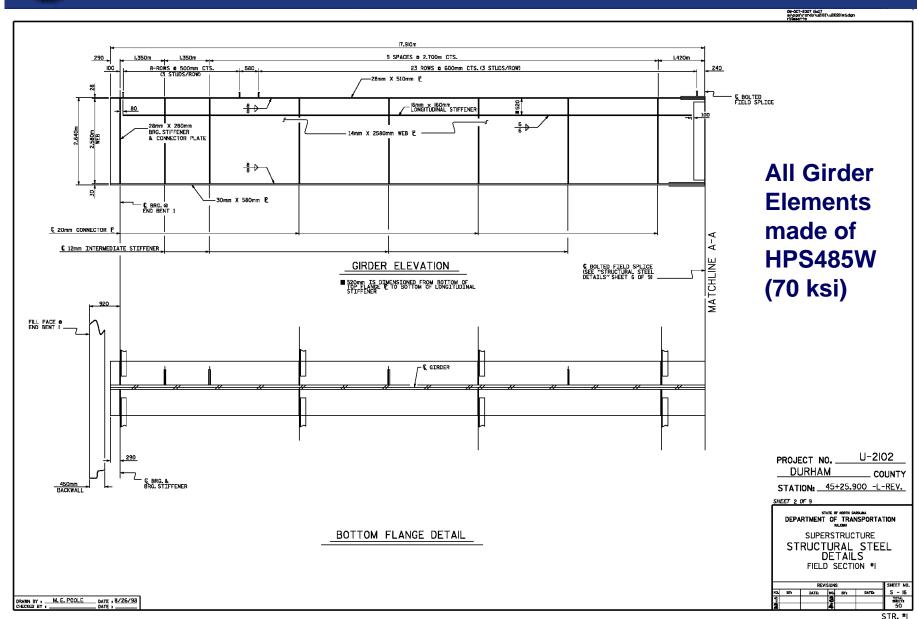
IN LIEU OF THE TURN OF THE NUT METHOD, TENSION ON A325M BOLTS MAY BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS, FOR DIRECT TENSION INDICATORS, SEE SPECIAL PROVISIONS.

ALL DIMENSIONS SHOWN ARE HORIZONTAL OR VERTICAL, UNLESS OTHERWISE NOTED.

ALL FIELD CONNECTIONS TO BE 22.23mm HIGH STRENGTH BOLTS UNLESS NOTED.

FOR SHEAR STUDS, SEE SPECIAL PROVISIONS.





Lessons Learned

- More efficient to use in a "hybrid" configuration
- Design to eliminate transverse and longitudinal stiffeners
- Carefully evaluate the material premium to ensure the most economical design

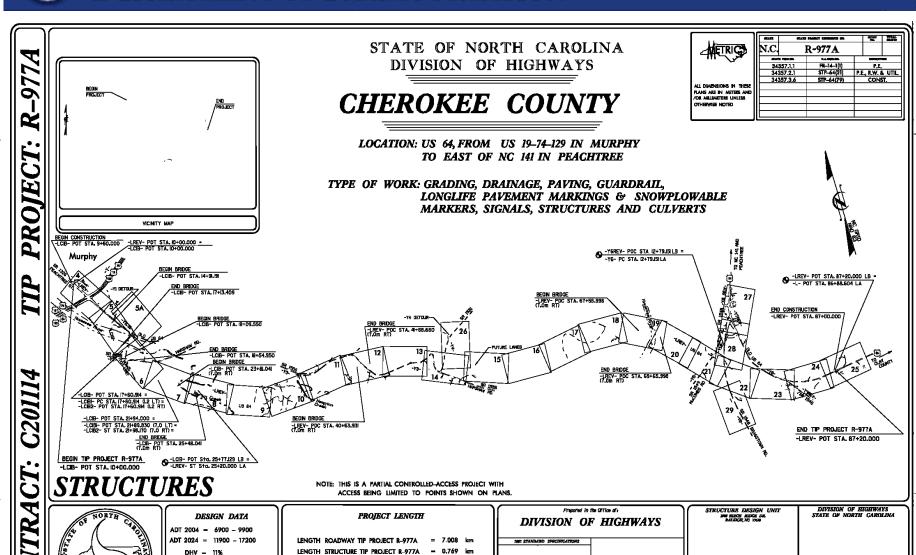


Designer: NCDOT SDU

Let: 3-15-2005

Contractor: Simpson Construction

Company (Structures)



= 7,777 km

LETTING DATE:

MARCH 15, 2005

L.E. SUTTON, P.E.

STATE ENGINEY DIGROUS - DESIGN DEPARTMENT OF TRANSPORTATION

TOTAL LENGTH TIP PROJECT R-977A

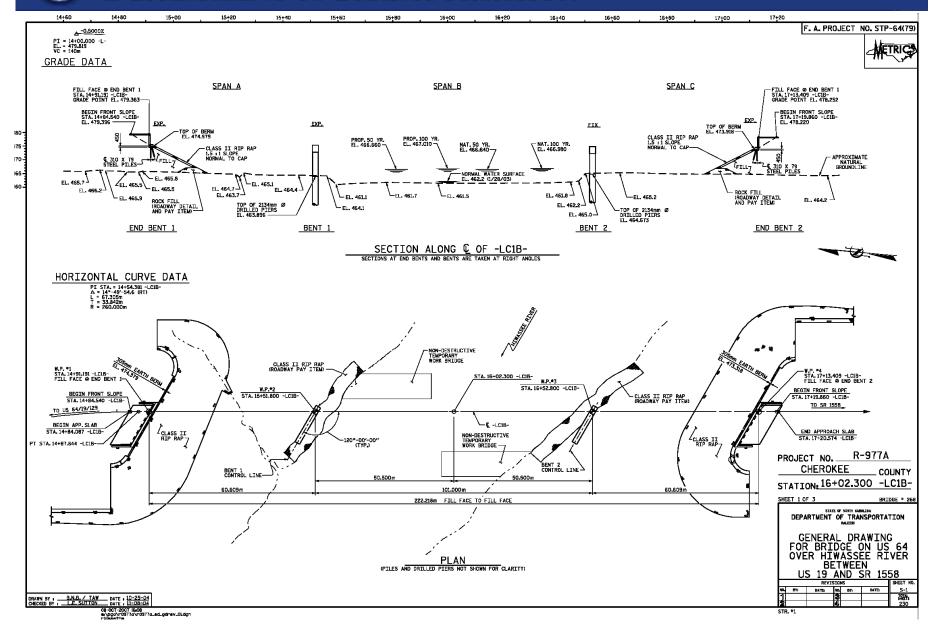
T = 4% [1% T1ST+3% DUALS] V = 80 km/h (-LC1B-)

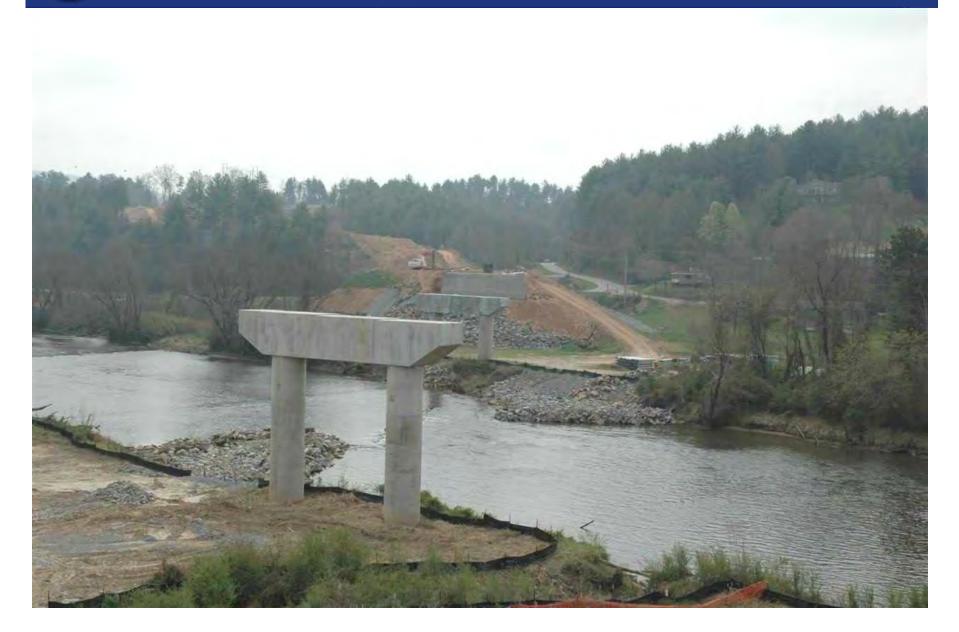
V = 100 km/h (-LREV--)

Project Data - Hiwassee River Span

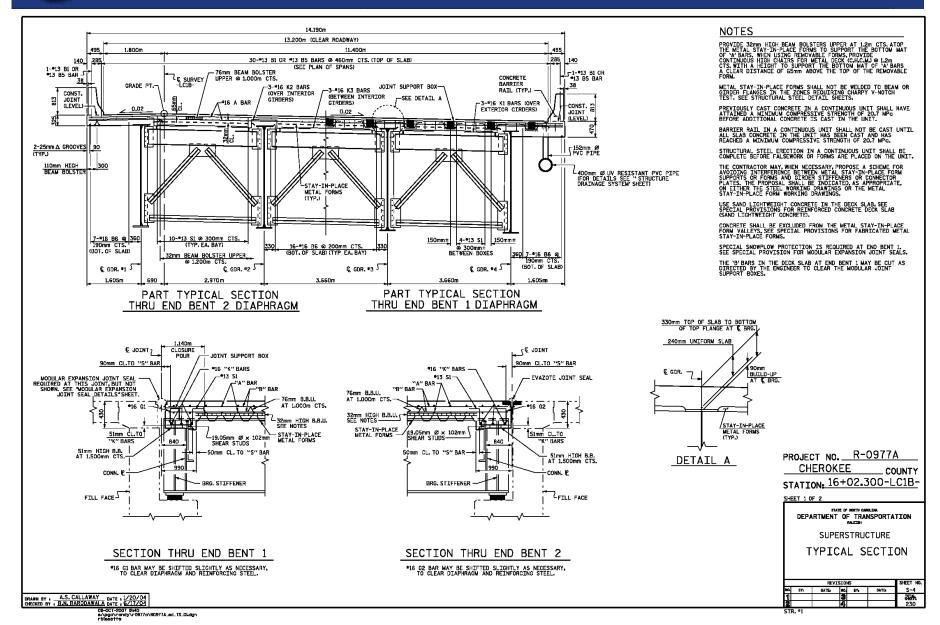
- 3 Span Continuous Hybrid Steel Girders
 - 101m (331') main with 60.6m (199') end spans
- Clear Roadway Width of 13.2m (43'-3")
- Girder Spacing of 3.660m (12')
- Girder Depth of 3m (9'-10")
- HPS485W (70 ksi) used in key locations to economize design

4

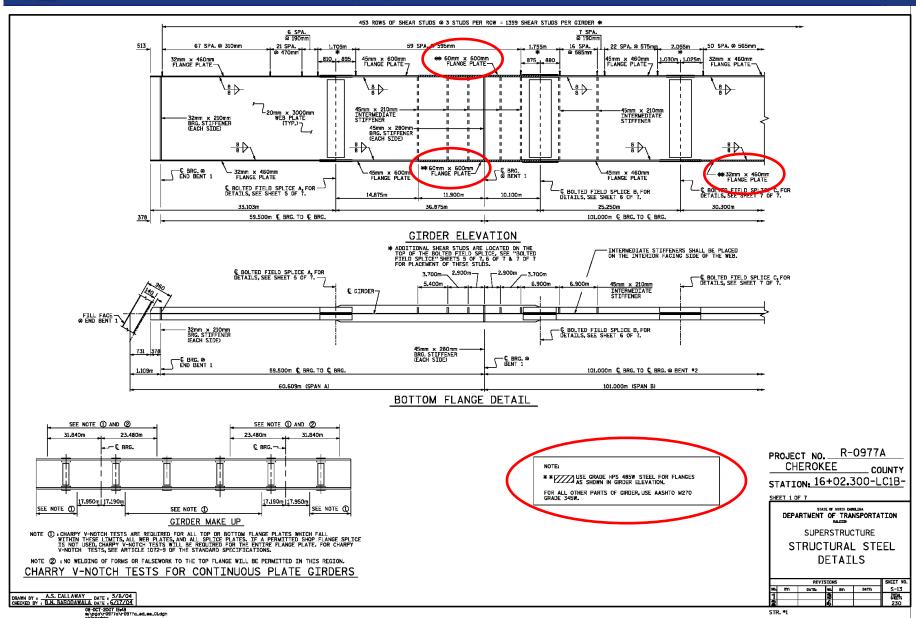




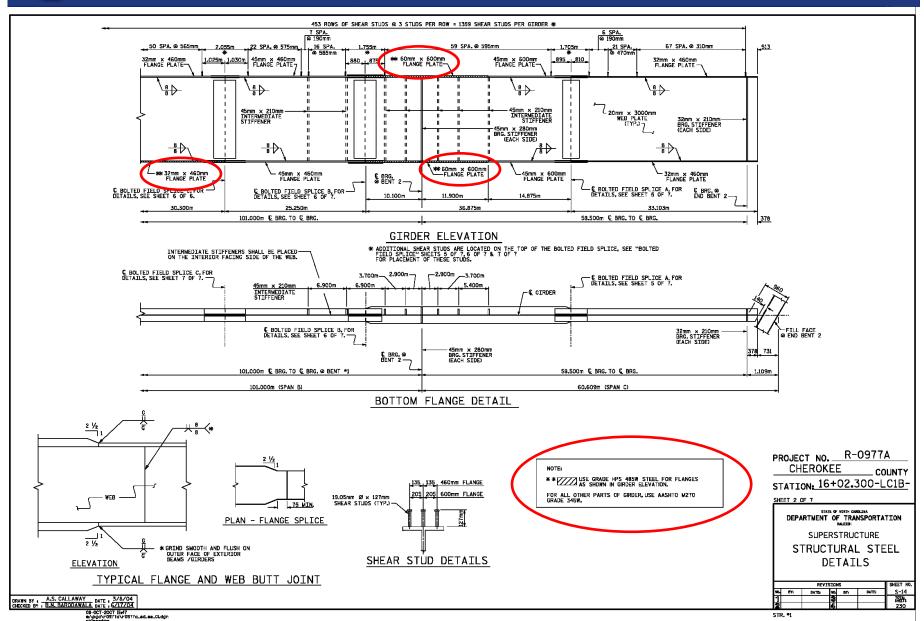


















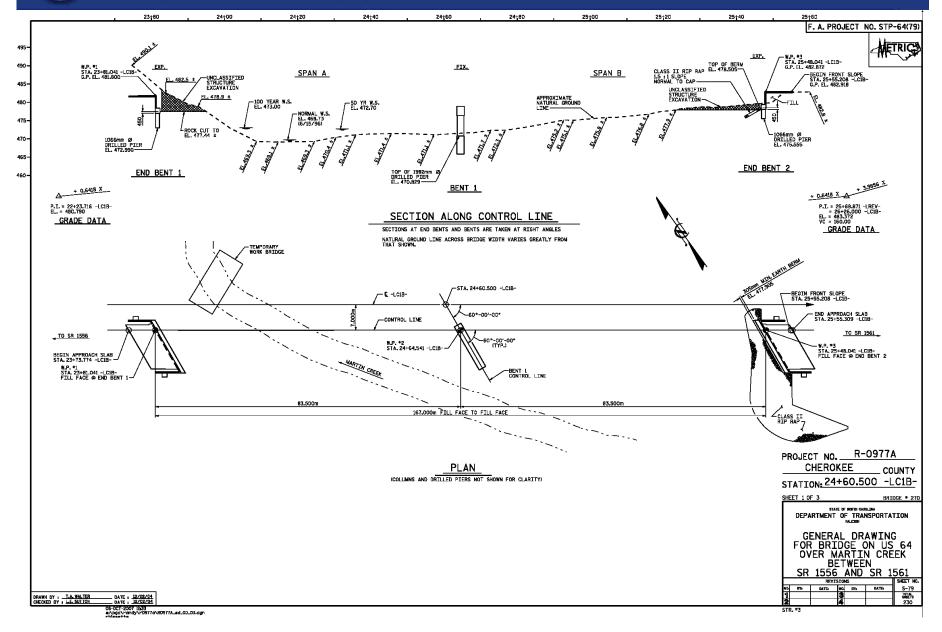




Project Data - Martin Creek Span

- 2 Span Continuous Hybrid Steel Girders
 - Equal 83.5m (274') spans
- Clear Roadway Width of 11.5m (37'-9")
- Girder Spacing of 3.36m (11')
- Girder Depth of 2.72m (8'-11")
- HPS485W (70 ksi) used in key locations to economize design

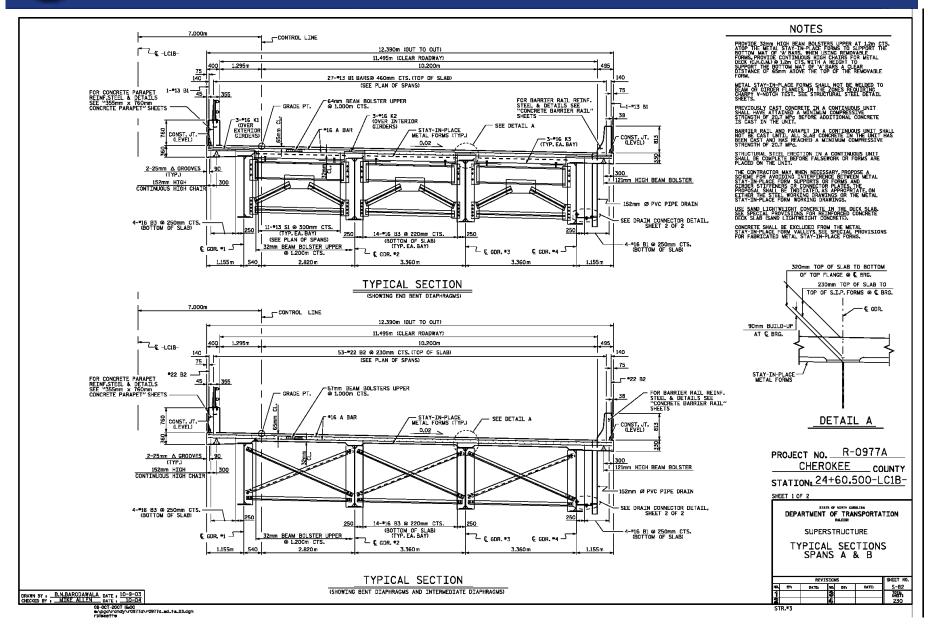




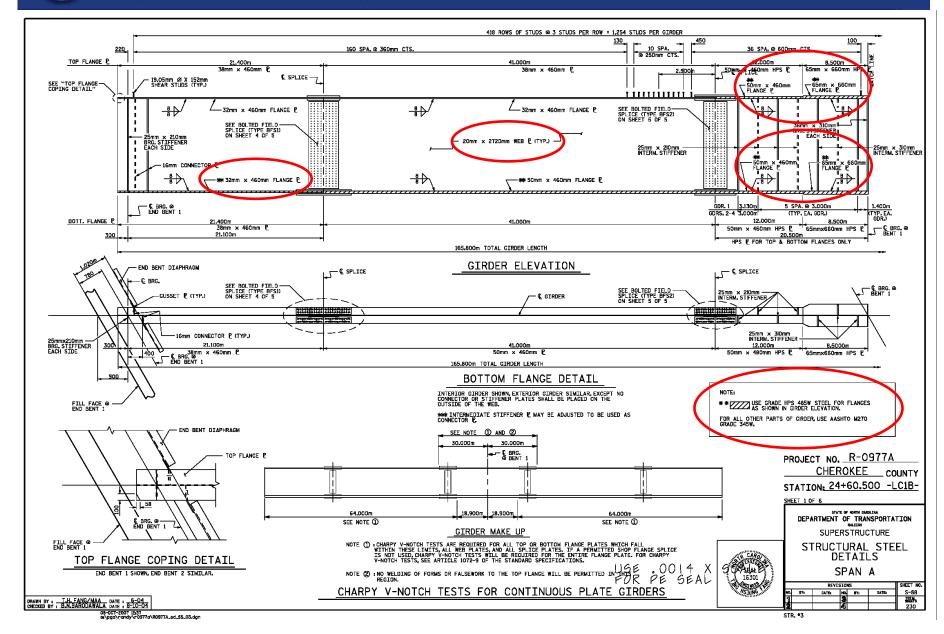




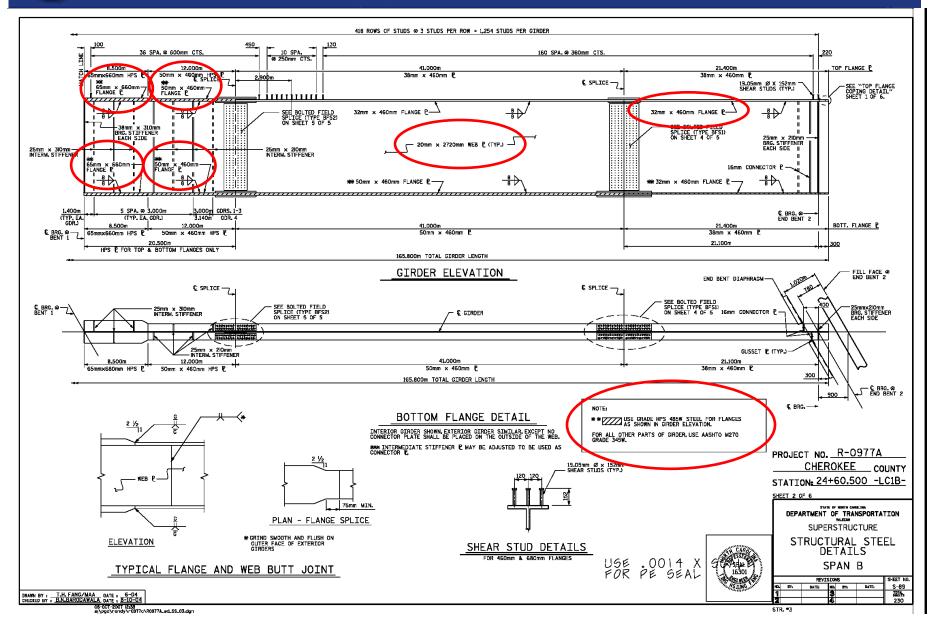
















Lessons Learned

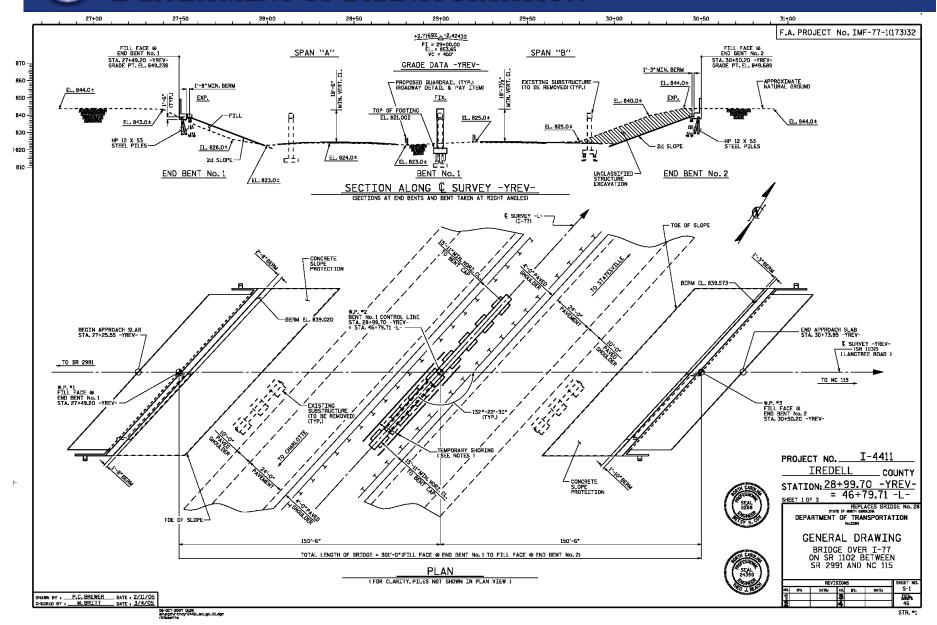
- Efficient use of "hybrid" configuration HPS70W & Grade 50W flange plates and Grade 50W web plate
- Design eliminates use of most transverse and all longitudinal stiffeners
- Economical section design
- Ideal situation



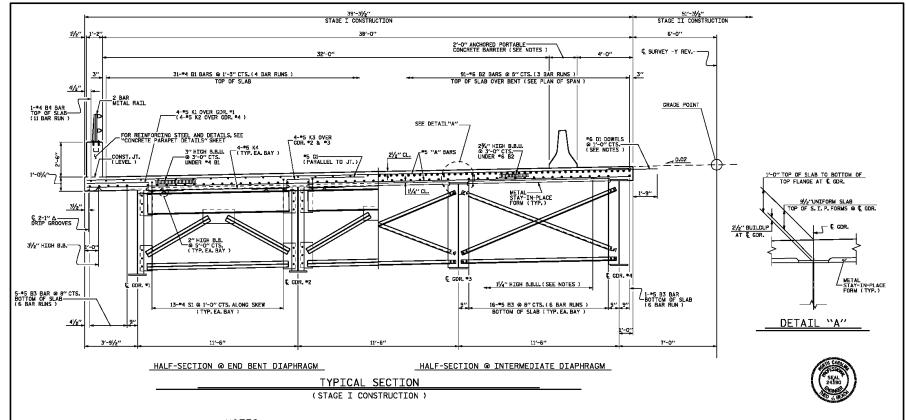
Project Data

- 2 Span Continuous Hybrid Steel Girders
 - Equal 150'-6" (45.9m) spans
- Clear Roadway Width of 88' (26.8m)
- Girder Spacing of 11'-6" (3.5m)
- Girder Depth of 5'-4" (1.6m)
- IBRD Funded project to evaluate use of HPS100W in NC

4







NOTES

PROVIDE 1/4*HIGH BEAM BOLSTERS UPPER AT 4*-0*CTS, ATOP THE METAL STAY-TH-PLACE FORMS TO SUPPORT THE BOTTOM MAT OF "A" BARS, WHEN USING REMOVABLE FORMS, PROVIDE CONTINUOUS HIGH CHAIRS FOR METAL DESK (C.H.C.M.) 0.4*-0*CTS, WITH A HEIGHT TO SUPPORT THE BOTTOM MAT OF "A" BARS A CLEAR DISTANCE OF 22/4 RADOVE THE TOP OF THE REMOVABLE FORM.

 $^{\rm +6}$ DI DOWELS SHALL BE PLACED IN THE SAME HORIZONTAL PLANE AS THE TOP SLAB REINFORCING STEEL.

METAL STAY-IN-PLACE FORMS SHALL NOT BE WELDED TO CIRDER FLANCES IN THE ZONES REQUIRING CHARPY V-NOTCH TEST. SEE STRUCTURAL STEEL DETAIL SHEETS. PREVIOUSLY CAST CONCRETE IN A CONTINUOUS UNIT SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT.

PARAPET IN A CONTINUOUS UNIT SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE UNIT HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

STRUCTURAL STEEL ERECTION IN A CONTINUOUS UNIT SHALL BE COMPLETE BEFORE FALSEWORK OR FORMS ARE PLACED ON THE UNIT.

"G" BARS MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO CLEAR REINFORCING STEEL AND STIRRUPS,

SEE TRAFFIC CONTROL PLANS FOR LOCATION AND PAY LIMITS OF THE ANCHORED PORTABLE CONCRETE BARRIER.

THE CONTRACTOR MAY, WHEN NECESSARY, PROPOSE A SCHEME FOR AVOIDING INTERFERENCE BETWEEN METAL STAY-IN-PLACE FORM SUPPORTS OR FORMS AND BEAM/GIRDER STIFFERENS OR CONNECTOR PLATES. THE PROPOSAL SHALL BE INDICATED, AS APPROPRIATE, ON EITHER THE SIELL WORKING DRAWINGS OR THE METAL STAY-IN-PLACE FORM WORKING DRAWINGS.

ALL REINFORCING STEEL IN PARAPETS SHALL BE EPOXY COATED.

THE NOMINAL UNCOMPRESSED SEAL WIDTH OF THE EVAZOTE JOINT SEAL SHALL BE

THE CONTRACTOR SHALL ADJUST THE GIRDER BUILDUPS AS NECESSARY TO INCORPORATE A MAXIMUM PERMISSIBLE VARIATION IN POT BEARING DEPTH OF $\frac{1}{12}$. SEE SPECIAL PROVISION FOR POT BEARINGS.

PROJECT NO. I-4411
IREDELL COUNTY
STATION: 46+79.71 -L-

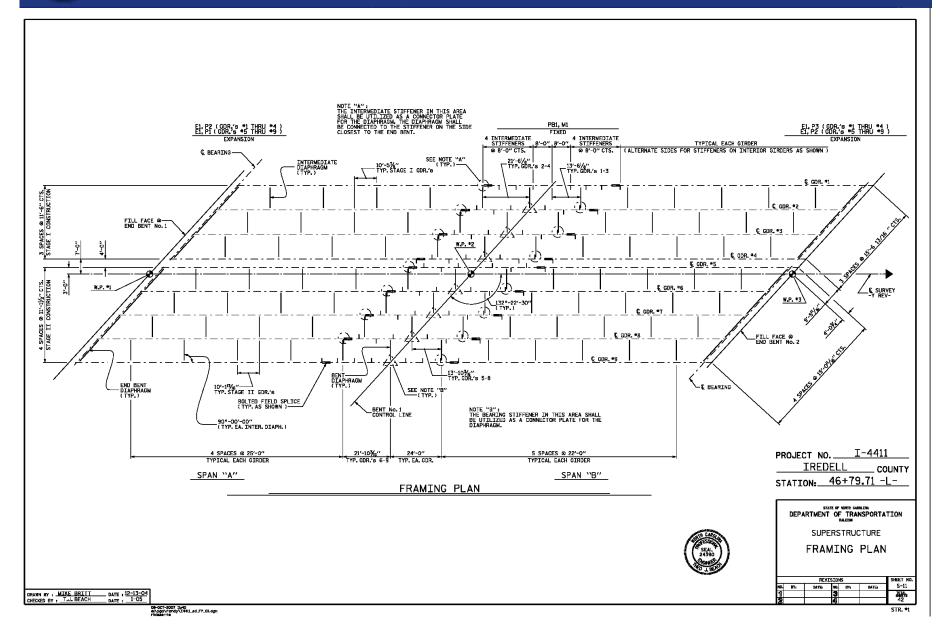
SHEET 1 OF 2

DEPARTMENT OF TRANSPORTATION

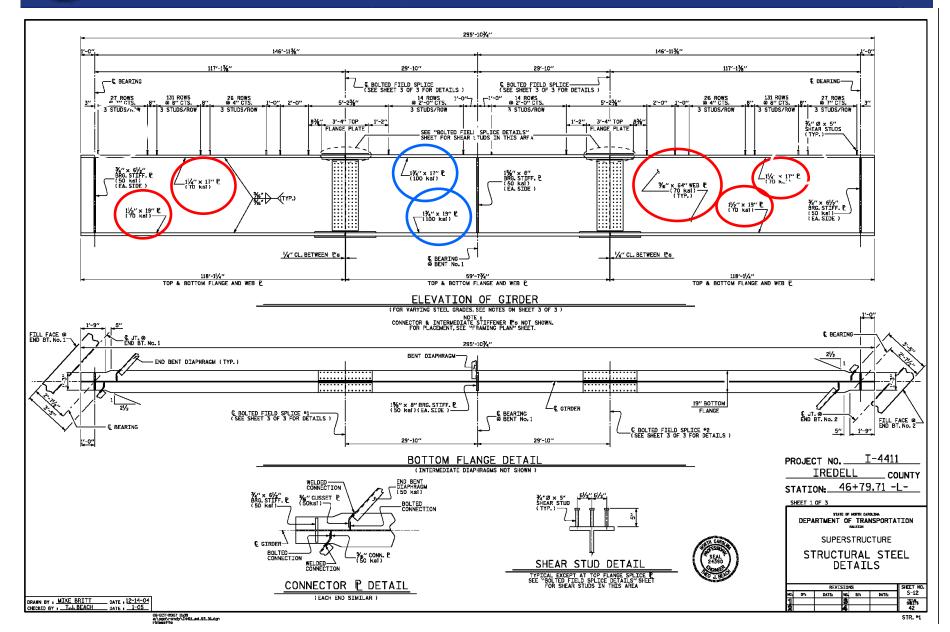
SUPERSTRUCTURE
TYPICAL SECTIONS

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Lessons Learned

- IBRD Funded Project using HPS100W/HPS70W hybrid section with Grade 50W miscellaneous plates
- Likely not ideal application of HPS100W, but used as trial material
- Further applications pending results of trial project

NCDOT HPS "Guidelines"

- No formal policy at present
- Use in hybrid configuration on long spans
- Strategically locate to economize steel sections
- Must result in overall cost savings especially on project with multiple structures

Questions?